Report of: Asphalt Mix Design and Testing Using Fox River Sediment Glass Aggregates

Minergy

December 22, 2003

OMNNI Prj. T0868A03-003

ENGINEERING • ARCHITECTURE • ENVIRONMENTAL



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REPORT OF: ASPHALT MIX DESIGN AND TESTING

PROJECT: Fox River Sediment Glass Aggregate

OMNNI PROJECT NO. T0868A03-003

Asphalt Mix Designs

CLIENT: Mr. Tom Bauduin

Minergy

1512 S. Commercial Street

Neenah, WI 54956

DATE: December 22, 2003

INTRODUCTION

This report presents the results of asphalt mix design and testing using glass aggregate produced by Minergy. Glass aggregate from the Minergy Winneconne Pilot Plant produced from sediments dredged from the Fox River was used in the design and testing. Two sizes of glass aggregates were used in the asphalt mix designs. One size has a gradation that is naturally produced by the plant during the incineration process. The other size consisted of glass aggregate crushed to meet ASTM: C33 gradation requirements for concrete fine aggregate. These glass aggregates were incorporated into an existing Wisconsin Department of Transportation (WisDOT) verified; 19 mm, E-1 asphalt mix currently being produced by a local asphalt mix supplier. The purpose of this testing and evaluation was to compare the physical properties of the asphalt paving mixtures containing a glass aggregate component to a similar mix composed exclusively of locally available, naturally occurring aggregates and recycled asphalt pavement (RAP). Both physical properties of the asphalt mixes and the effect of moisture on the ability of the asphalt binder to adhere to the aggregate of the mixtures containing the glass aggregate were compared to the traditional asphalt mix design obtained from the asphalt supplier and the Wisconsin Department of Transportation specifications.

The scope of our work consisted of the following tasks:

- Mobilize technicians to the asphalt plant site located in the Green Bay area to obtain aggregate and recycled asphalt pavement (RAP) to be used in performing asphalt mix designs. Obtain asphalt binder from the asphalt binder supplier.
- Perform two asphalt mix designs (WisDOT 1559-01) using aggregate blends containing a Fox River sediment glass aggregate component in a mixture with locally available, naturally occurring aggregates and recycled asphalt pavement (RAP). One of the designs would incorporate the glass aggregate that has a gradation that is naturally produced by the plant during the incineration process. The second design would use glass aggregate that has been crushed to a finer gradation, meeting the ASTM: C33 concrete fine aggregate grading specification. The mix design aggregate proportions would be based on an existing asphalt mix design that uses the same naturally occurring aggregates and RAP. The mix designs will include aggregate gradations, bulk specific gravities and absorptions, elongated particles, fine aggregate angularity, sand equivalency, % fractured faces in the coarse aggregate, bulk mix density (Gmb), maximum theoretical specific gravity (Gmm), total voids, voids in mineral aggregate (VMA), voids filled with binder (VFB), dust/binder ratio, and tensile strength ratio testing.
- Compare the physical properties of an asphalt mix using the glass aggregate to the existing

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> mix design that did not include the glass aggregate. Also compare the effect of moisture on the ability of the asphalt binder to adhere to the aggregate of the mixture containing the glass aggregate to the existing mix design and the Wisconsin Department of Transportation specifications.

· Prepare a report summarizing our findings.

MATERIALS

A local asphalt mix producer's 19 mm, E-1 mix design was used to incorporate the glass aggregates into and to provide a standard asphalt mix to which mix properties could be compared. A copy of the mix design report is included in Appendix B. The mix components as listed on this design were used to perform our work. The following materials were used in designing and testing the asphalt mixes:

 Aggregates: 7/8" x 5/8" Chip 5/8" x ½" Chip

1/2" x 1/4" Chip

Washed Manufactured Sand

Natural Sand

Crushed Recycled Asphalt Pavement (RAP)

Glass Aggregate - Minergy, Winneconne (Fox River

sediments)

Asphalt Binder PG 58-28 – Koch Pavement Solutions, Green Bay

Terminal

Naturally occurring aggregates, recycled asphalt pavement (RAP) and asphalt binder were supplied by an asphalt mix supplier located in the Green Bay area. Aggregates and RAP were sampled from the plant stockpiles and the asphalt binder was obtained from Koch Pavement Solutions, who had sampled it from the Green Bay terminal. The crushed stone chips and manufactured sand are produced from Silurian age dolomitic limestone from the Niagara Formation in Brown County, Wisconsin. The natural sand is composed predominantly of quartz sand and is mined from a glacial outwash deposit in Brown County, Wisconsin. Minergy personnel delivered glass aggregates from the Winneconne pilot plant to our laboratory.

The naturally occurring aggregates used in the asphalt supplier's mix design were obtained from Wisconsin DOT qualified aggregate sources. Aggregate sources are qualified through the WisDOT Central Laboratory in Madison and the aggregate qualification typically involves the DOT testing them for Sodium Sulfate Soundness (AASHTO T104), LAR Wear (AASHTO T96) and Freeze-Thaw (AASHTO T103). The glass aggregate provided for this work has not been qualified by the WisDOT Central Laboratory. However, the testing for soundness, wear and freeze-thaw are performed on coarse aggregates only and would not be required for the glass

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aggregate for it to be used as a fine aggregate (passing the #4 sieve) component in a WisDOT verified mix design.

TEST METHODS

Asphalt mixes were designed to meet the current Wisconsin Department of Transportation's requirements as specified in section 460 of the WisDOT Standard Specifications for Highway and Structure Construction, 2003 Edition. Aggregate and asphalt mix tests were performed in accordance with test methods specified by WisDOT Procedure 1559-01.

The glass aggregates generally consisted of very angular, sand sized particles with very little passing the #200 sieve. Glass aggregate test reports have been included in Appendix D.

Aggregate from the naturally occurring aggregates and RAP sampled from the asphalt supplier's plant stockpiles were tested for gradation, specific gravity and fine aggregate angularity. Asphalt binder content of the RAP was also determined. All test results were compared to the results reported on the asphalt supplier's mix design report to verify that they were reasonably close to the aggregate components used during the design.

To assess the effect of using glass aggregate as a mix aggregate component, OMNNI Associates mix design #903003 was performed using the asphalt supplier's mix design and substituting the washed manufactured sand with the uncrushed glass aggregate. The washed manufactured sand was replaced because the gradation and angularity was most similar to the glass aggregates. OMNNI Associates mix design #902903 was performed using the asphalt supplier's mix design and substituting the washed natural sand with the crushed glass aggregate. For each mix design, a series of test blends were prepared in the laboratory to determine the aggregate blend percentages that would result in a mix that contained approximately 13-14% Voids in Mineral Aggregate (VMA) at the predicted optimum asphalt binder content. When this blend percentage was determined, further laboratory batching and testing was performed for each mix design to determine the optimum asphalt binder content, %G_{mm} at N_{initial} and %G_{mm} at N_{max}, and Tensile Strength Ratio (TSR). Mix design reports were prepared and are included in Appendix C.

The TSR test was also performed on the existing contractor's mix design using aggregates and asphalt binder from the current sampling so that a direct comparison with the glass aggregate mix designs could be obtained. The aggregates and asphalt binder were blended using the blend percentages reported in the original mix design report.

A summary of all test results is included in Appendix A.

DISCUSSION AND CONCLUSIONS

Test results indicate that the substitution of the washed manufactured sand and washed natural sand with the glass aggregates generally resulted in an increase in the amount of recycled asphalt

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pavement (RAP) that could be included in the mix and a decrease in the amount of natural and manufactured sands needed. It is our opinion that this is because the high angularity of the glass aggregate helps to build voids in mineral aggregate (VMA) into the mix, allowing the addition of more fines (passing #200 sieve) and requiring less sand to be used in the mix.

Because the aggregates substituted into the asphalt supplier's existing asphalt mix are considered to be glass, the possibility of an increased susceptibility to stripping was an item for which we tested using each of the glass aggregates. The Tensile Strength Ratio (TSR) test is the test that is currently used by the Wisconsin DOT to predict the susceptibility of the asphalt binder to strip away from the aggregates when subjected to the presence of water and general weathering. TSR results for the mixes using the glass aggregates were 81 for design #902903 and 78 for design #903003, which are very close to the TSR result of 78 obtained from the mix consisting of aggregates and asphalt binder that were blended in accordance with the asphalt producer's mix design report. These test results indicate that the mixes containing the glass aggregates are also not susceptible to stripping.

Based on the results of our testing, it is our opinion that the Fox River sediment glass aggregate included in these asphalt mixes can be successfully used as an aggregate component in WisDOT verified asphalt mix designs as well as commercial asphalt mix designs. The grading and high angularity of the glass aggregate helps to build VMA into the mix, allowing for an increased percentage of RAP and a decrease in the percentages of natural and manufactured sands used in the mix, which should produce a highly economical asphalt mix. Prior to full-scale use of asphalt mix containing a glass aggregate, producers and users may elect to produce the mix and place it in a test area where it can be monitored for long term susceptibility to stripping, and/or other concerns that they may have. However, the results of all testing performed on these glass aggregate mixes to-date indicate that the mixes will perform very well.

If you have any questions regarding this report, please contact us.

Sincerely,

Paul R. Eggen

Program Manager - Materials Testing

Appendix A

Asphalt Aggregate and Mix Properties Summary



ASPHALT AGGREGATE AND MIX PROPERTIES SUMMARY

Minergy, Prj. T0868A03-003

	Asphalt Mix Supplier's Mix Design	Mix Using Crushed Glass Agregate Design #902903	Mix Using Uncrushed Glass Agregate Design #903003	WisDOT Section 460 Specification 19 mm, E-1
Mix Blend Percentages				
7/8"x 5/8" Chip (%)	16	20	20	
5/8"x 1/2" Chip (%)	11	10	10	
1/2"x 1/4" Chip (%)	5	6	10	
Washed Man Sand (%)	30	25		
Natural Sand (%)	26	*	23	
RAP (%)	12	14	15	
Uncrushed Glass Aggregate (%)			22	
Crushed Glass Aggregate (%)	,	22		
Asphalt Binder - Added (%)	4.10	3.95	3.70	
Asphalt Binder - Total (%)	4.50	4.63	4.43	
Blended Aggregate Properties	3			
1" (25.0 mm)	100	100	100	100
3/4" (19.0 mm)	100	6'66	6.66	90-100
1/2" (12.5 mm)	6.68	6.68	6.68	90 max
3/8" (9.5 mm)	76.4	73.3	73.2	
#4 (4.75 mm)	61.5	57.3	9.99	
#8 (2.36 mm)	45.2	45.2	45.4	23-49
#16 (1.18mm)	33.7	30.2	28.7	
#30 (0.60mm)	25.1	17.7	20.4	
#50 (0.30mm)	14.7	10.4	12.1	
#100 (0.15mm)	6.5	6.3	5.5	
#200 (0.075mm)	4.6	4.6	4.1	2-8



ASPHALT AGGREGATE AND MIX PROPERTIES SUMMARY

Minergy, Prj. T0868A03-003 December 22, 2003

	Asphalt Mix Supplier's Mix Design	Mix Using Crushed Glass Agregate Design #902903	Mix Using Uncrushed Glass Agregate Design #903003	WisDOT Section 460 Specifications 19 mm, E-1
Fine Aggregate Angularity (%)	45.7	46.2	45.3	40 Min
Bulk Specific Gravity (GSB)	2.736	2.765	2.753	
Apparent Specific Gravity (GSA)	2.814	2.834	2.816	
Effective Specific Gravity (GSE)	2.788	2.828	2.805	
Absorption		0.91	0.91	
Mix Properties				
Total Voids	4.0	4.0	4.0	4.0
Voids in Mineral Aggregate (VMA)	13.3	13.4	13.1	13.0 Min
Voids Filled w/ Binder (VFB)	6.69	70.1	69.5	65-78
Maximum Specific Gravity (Gum)	2.589	2.617	2.606	
Bulk Specific Gravity (Gmb)	2.485	2.512	2.502	
% Gmm @ Ninitial	90.3	6.68	91.0	< 90.5(11)
% G _{mm} @ N _{max}	96.3	2.96	6.96	≥ 98.0
Tensile Strength Ratio (TSR)	752	81	78	70 Min
Dust to Effective Binder Ratio	1.2	1.2	1.1	1.2 Max
Film Thickness Estimate (Microns)		7.77	77.7	

^[1] The percent maximum density at initial compaction is only a guideline.
[2] A TSR test result of 78 was obtained by OMNNI Associates using aggregates and asphalt binder sampled for mix designs #902903 and #903003.

Appendix B

Asphalt Mix Supplier's 19 mm, E-1 Mix Design

REPORT OF SUPERPAVE VOLUMETRIC ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

TYPE E-1, 19 mm DESIGN NUMBER: Supplier Design DESIGN ESALS . . 1.0E+06

JOB NUMBER

AGGREGATE SOU	IRCE								
SPL NO.	TEST NO.		MATERIA	L					
1	A-55-02		7/8" x 5/8"	Chip					
2	A-56-02		5/8" x 1/2"	Chip					
3	A-57-02		1/2" x 1/4"	Chip					
4	A-67-02		Washed M	anufactured S	and				
5	A-60-02		Natural Sa	nd					
6	B-7-02		Crushed R	AP					
AGGREGATE GRA	DATION (9	% PASSING):						
SPL NO.	1	2	3	4	5	6			
BLEND %	16	11	5	30	26	12	BLEND	JOB MIX	SPEC
25mm (1.0*)	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0
19mm (3/4*)	99.7	100.0	100.0	100.0	100.0	100.0	100.0	100.0	90-100
12.5mm (1/2*)	53.2	76.5	100.0	100.0	100.0	100.0	89.9	89.9	0-90
9.5mm (3/8*)	12.5	20.6	92.2	100.0	100.0	96.1	76.4	76.4	-
4.75mm (#4)	1.9	4.8	9.9	88.6	93.2	78.2	61.5	61.5	
2.36mm (#8)	1.9	3.7	5.0	53.0	80.3	62.5	45.2	45.2	23-49
1.18mm (#16)	1.8	3.1	4.3	29.3	69.2	50.3	33.7	33.7	***
0.60mm (#30)	1.8	2.8	4.0	15.9	56.3	40.8	25.1	25.1	***
0.30mm (#50)	1.8	2.7	3.9	9.4	28.8	29.7	14.7	14.7	
0.15mm (#100)	1.8	2.6	3.8	5.8	5.9	20.4	6.5	6.5	
0.075mm (#200)	1.6	2.4	3.4	3.0	2.6	14.9	4.1	4.1	2-8
G _{sb}	2.740	2.743	2.708	2.790	2.697	2.693	2.736		
FAA	1000000000		-	48.7	41.4	47.4			
FOR JMF AS USE	D IN LABO	RATORY T	RIAL MIX	ES:					
SOUNDNESS	=	0.5				ELONGATE	PARTICLES =		1%
L.A. WEAR =	3.8	(100)	28.9	9 (500)			TED VOIDS - F.		45.7%
FREEZE/THAW	=			(000)		BULK AGG.		0	2.736
SAND EQUIVALEN		91.7				And the second second second second second	AGG, SP, GR. =		2.814
CRUSH	=	95.8/1F, 98	8.5/2F				AGG. SP. GR.		2.788
BINDER DATA						RAP DATA			
TYPE	PG 58-28						1, D1856, D5)		
SOURCE						% BINDER			3.54
SP. GR. (77/77F) .									
AVERAGE TEST D	DATA	(60 GYRA							
BINDER CONT.	****	- % 'VOIDS		TH. MAX.	BULK				
(% OF MIX)	MIX	VMA	VFB	SP. GR.	SP. GR.				
ADDED				(G _{mm})	(G _{mb})				
3.70	5.3	13.5	60.7	2.606	2.467				
4.20	3.7	13.2	72.0	2.585	2.490				
4.70	2.0	12.8	84.4	2.565	2.513				
12/12/11									

THE TEST DATA SHOWN ON THIS REPORT PERTAIN ONLY TO THE MATERIAL SUBMITTED FOR DESIGN AASHTO PP28 MODIFIED TO MEET CUSTOMER REQUIREMENTS

2.524

2.545

5.20

0.8

12.9

93.8

REPORT OF SUPERPAVE VOLUMETRIC ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

TYPE E-1, 19 mm DESIGN ESALS . . 1.0E+06 DESIGN NUMBER: Supplier Design

JOB NUMBER

FOR:

N_{initial} 7 N_{design} 60

75

Nmax

RECOMMENDED OPTIMUM BINDER CONTENT =

4.10 % ADDED

4.50 % TOTAL

DESIGN DATA AT OPTIMUM BINDER CONTENT:

MIX VOIDS (%) 4.0 = VMA (%) 13.3 VFB (%) 69.9 TH. MAX. SP. GR. (Gmm) 2.589 BULK SP. GR. (Gmb) 2.485 UNIT WEIGHT (lbs/ft3) 154.7 %Gmm @ Ninitial 90.3 %Gmm @ Nmax 96.3 TSR N= 20 75.2 RECOMMENDED MIXING TEMP. = 275-300 DUST TO Pbe RATIO 1.2

COMMENTS:

THE TEST DATA SHOWN ON THIS REPORT PERTAIN ONLY TO THE MATERIAL SUBMITTED FOR DESIGN PAGE 2

Appendix C

OMNNI Glass Aggregate, 19 mm, E-1 Mix Designs #902903 and #903003

4.17

4.67

2.3

1.0

12.7

12.7

82.0

91.8

2.587

2.567



REPORT OF SUPERPAVE ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

TYPE E1-19.0 mm DESIGN NUMBER: 903003
DESIGN ESALS ... 1.0E+06
JOB NUMBER ... T0868A03-003 DATE: 12/11/2003

JOB NOWIDEN	. 10000AUG	5-005						DATE:	12/11/2
AGGREGATE SOL	JRCE								
SPL NO.	TEST NO		MATERIA	AL.					
1	24790		7/8 x 5/8 (
2	24890		5/8 x 1/2 C						
3	24990		1/2 x 1/4 C						
4	25190		Washed N	0.7					
5	25490		Glass Agg						
6	25290		Crushed R						
AGGREGATE GRA	ADATION (% PASSIN	G):						
SPL NO.	1	2	3	4	5	6			
BLEND %	20	10	10	23	22	15	BLEND	JOB MIX	SPEC.
25mm (1.0")	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100
19mm (3/4")	99.3	100.0	100.0	100.0	100.0	100.0	99.9	99.9	90-100
12.5mm (1/2")	60.5	80.8	100.0	100.0	100.0	98.1	89.9	89.9	0-90
9.5mm (3/8")	12.9	20.1	95.9	100.0	100.0	93.4	73.2	73.2	
4.75mm (#4)	3.3	4.3	5.2	93.9	99.9	76.3	56.6	56.6	
2.36mm (#8)	2.9	3.6	3.6	76.9	79.6	59.3	45.4	45.4	23-49
1.18mm (#16)	2.7	3.2	3.4	64.2	26.3	46.6	28.7	28.7	
0.60mm (#30)	2.6	3.1	3.3	52.1	7.9	37.1	20.4	20.4	***
0.30mm (#50)	2.5	3.0	3.2	27.3	2.5	27.5	12.1	12.1	***
0.15mm (#100)	2.5	2.9	3.1	6.0	0.7	18.9	5.5	5.5	***
0.075mm (#200)	2.2	2.6	2.8	2.8	0.1	13.2	3.6	4.1	2-8
G _{sb}	2.751	2.746	2.736	2.680	2.878	2.698	2.753		
Absorption (%)	1.07	1.09	1.29	1.19	0.30	0.79	0.91		
FAA		2019/201		41.9	50.5	42.9	17.5		
FOR JMF AS USE	D IN LABO	RATORY	TRIAL MIX	ES:					
SOUNDNESS	=	0	(217-24-2	70 Part (1)		ELONGATED PA	RTICLES =	0.0%	
L.A. WEAR =	4.9	(100)		3 (500)		UNCOMPACTED		45.3	
FREEZE/THAW	=	0.2	- 710	TO STOTE OF		BULK AGG. SP. G		2.753	
SAND EQUIVALEN		88				APPARENT AGG		2.820	
CRUSH	=	1F/100 2	F/99			EFFECTIVE AGG		2.805	
TYPE	PG 58-28	7				RAP DATA (ASTM D2171, D1	1856, D5)		
SOURCE						% BINDER		= 4.87	
AVERAGE TEST D	DATA	The second second	ATIONS AT	LDLVCD-CCCC					
BINDER CONT.	****	- % 'VOIDS		TH. MAX.	BULK				
(% OF MIX)	MIX	VMA	VFB	SP. GR.	SP. GR.				
ADDED				(G _{mm})	(G _{mb})				
3.17	5.7	13.4	57.8	2.629	2.480				
3.67	4.1	13.2	68.8	2.608	2.501				
4.47	0.0	40.7	00.0	0.503	0.000				

THE TEST DATA SHOWN ON THIS REPORT PERTAIN ONLY TO THE MATERIAL SUBMITTED FOR DESIGN AASHTO PP28 MODIFIED TO MEET CUSTOMER REQUIREMENTS

2.528

2.540

REPORT OF SUPERPAVE ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

TYPE E1-19.0 mm DESIGN NUMBER: 903003
DESIGN ESALS . . 1.0E+06

JOB NUMBER T0868A03-003 DATE: 12/11/2003

FOR: N_{initial} 7 N_{design} 60 N_{max} 75

RECOMMENDED OPTIMUM BINDER CONTENT = 3.70 % ADDED 4.43 % TOTAL

DESIGN DATA AT OPTIMUM BINDER CONTENT:

MIX VOIDS (%) 4.0 13.1 VMA (%) VFB (%) 69.5 TH. MAX. SP. GR. (G_{mm}) 2.606 BULK SP. GR. (Gmb) 2.502 UNIT WEIGHT (lbs/ft3) 155.8 %Gmm @ Ninitial 91.0 %G_{mm} @ N_{max} 96.7 N= 18 78 RECOMMENDED MIXING TEMP. = 275-300DUST TO Pbe RATIO 1.1

COMMENTS:

P-0.075 INCREASED 0.5% TO SIMULATE AGGREGATE BREAKDOWN DURING MIX PRODUCTION.

OMNNI ASSOCIATES

PAUL R. EGGEN

WISCONSIN CERTIFIED ASPHALTIC TECHNICIAN III

THE TEST DATA SHOWN ON THIS REPORT PERTAIN ONLY TO THE MATERIAL SUBMITTED FOR DESIGN PAGE 2

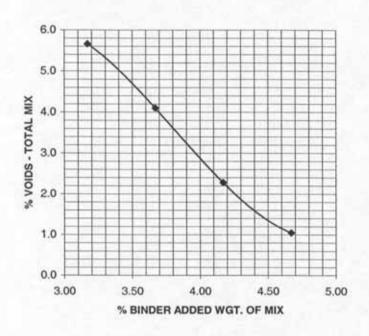


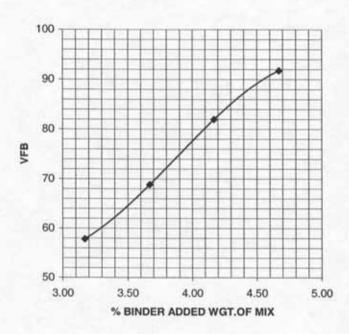
REPORT OFSUPERPAVE ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

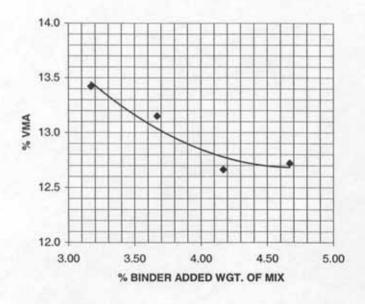
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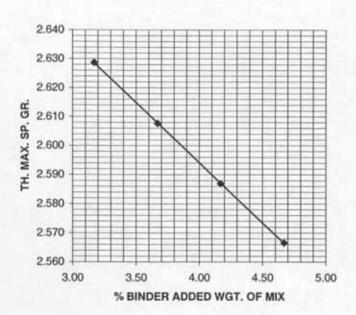
DESIGN ESALS. . . 1.0E+06

JOB NUMBER T0868A03-003 DATE: 12/11/03











REPORT OFSUPERPAVE ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

TYPE

E1-19.0 mm

1.0E+06

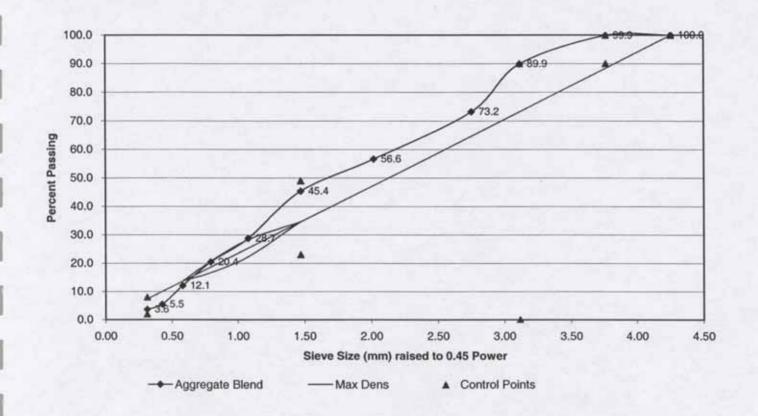
DESIGN ESALS... JOB NUMBER

T0868A03-003

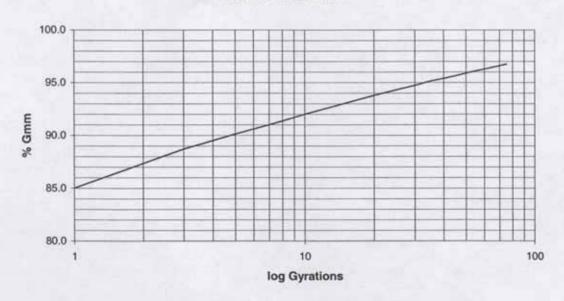
DESIGN NUMBER:

903003

DATE: 12/11/03



COMPACTION CURVE





REPORT OF SUPERPAVE ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

 TYPE......E1-19.0 mm
 DESIGN NUMBER:
 902903

 DESIGN ESALS...1.0E+06
 902903
 DATE:
 12/211/03

JOB NUMBER	T0868A03	3-003						DATE:	12/211/
AGGREGATE SO	URCE								
SPL NO.	TEST NO		MATERIA	L					
1	24790		7/8 x 5/8 C	hip					
2	24890		5/8 x 1/2 Ch	nip					
3	24990		1/2 x 1/4 Ct	nip					
4	25090		Washed Mf	g'd Sand					
5	25390			ass Aggregat	е				
6	25290		Crushed RA	AP					
AGGREGATE GF	RADATION (% PASSING	3):						
SPL NO.		2	3	4	5	6			
BLEND %	20	10	9	25	22	14	BLEND	JOB MIX	SPEC.
25mm (1.0")	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100
19mm (3/4")	99.3	100.0	100.0	100.0	100.0	100.0	99.9	99.9	90-100
12.5mm (1/2")	60.5	80.8	100.0	100.0	100.0	98.1	89.9	89.9	0-90
9.5mm (3/8")	12.9	20.1	95.9	100.0	100.0	93.4	73.3	73.3	**
4.75mm (#4)	3.3	4.3	5.2	92.3	100.0	76.3	57.3	57.3	***
2.36mm (#8)	2.9	3.6	3.6	57.4	96.6	59.3	45.2	45.2	23-49
1.18mm (#16)	2.7	3.2	3.4	33.6	64.3	46.6	30.2	30.2	
0.60mm (#30)	2.6	3.1	3.3	19.8	29.2	37.1	17.7	17.7	***
0.30mm (#50)	2.5	3.0	3.2	12.2	11.0	27.5	10.4	10.4	***
0.15mm (#100)	2.5	2.9	3.1	7.6	3.1	18.9	6.3	6.3	***
0.075mm (#200)	2.2	2.6	2.8	4.5	1.0	13.2	4.1	4.6	2-8
G _{sb}	2.751	2.746	2.736	2.727	2.886	2.698	2.765		
Absorption (%)	1.07	1.09	1.29	1.40	0.06	0.79	0.91		
FAA					48.0	42.9			
FOR JMF AS US	ED IN LABO	RATORY	TRIAL MIXE	· S-					
SOUNDNESS	ED III EADO	0.5	THAL MILAL			ELONGATED PA	BTICLES =	0.0%	
L.A. WEAR =		3 (100)	28.0	(500)		UNCOMPACTED		46.2	
FREEZE/THAW	=	0	20.0	(000)		BULK AGG. SP. 0	CONTRACTOR STATE OF THE PARTY O	2.765	
SAND EQUIVALE		76				APPARENT AGG		2.834	
CRUSH	=	1F/100 2F	F/100			EFFECTIVE AGG		2.828	
BINDER DATA						RAP DATA			
TYPE	. PG 58-28					(ASTM D2171, D	1856 D5\		
SOURCE	. Koch	2				% BINDER	1000, 00)	= 4.87	
SP. GR. (77/77F)						70 DINDEN		- 4.07	
AVERAGE TEST		(75 GYR/	ATIONS AT	275F)					
BINDER CONT.		- % 'VOIDS		TH. MAX.	BULK				
(% OF MIX)	MIX	VMA	VFB	SP. GR.	SP. GR.				
ADDED				10 1					

AVENAGE TEST D	MIM	(13 GINA	HONON	21011	
BINDER CONT.		% 'VOIDS		TH. MAX.	BULK
(% OF MIX)	MIX	VMA	VFB	SP. GR.	SP. GR
ADDED				(G _{mm})	(G _{mb})
3.62	5.1	13.6	62.6	2.631	2.497
4.12	3.4	13.2	74.3	2.609	2.521
4.62	1.9	13.0	85.4	2.589	2.539
5.12	1.0	13.4	92.5	2.568	2.542

THE TEST DATA SHOWN ON THIS REPORT PERTAIN ONLY TO THE MATERIAL SUBMITTED FOR DESIGN AASHTO PP28 MODIFIED TO MEET CUSTOMER REQUIREMENTS

REPORT OF SUPERPAVE ASPHALTIC MIX DESIGN

(AASHTO PP-28, ASTM D4867)

TYPE E1-19.0 mm DESIGN NUMBER: 902903
DESIGN ESALS . . 1.0E+06

JOB NUMBER T0868A03-003 DATE: 12/211/03

FOR: N_{initial} 7 N_{design} 60

RECOMMENDED OPTIMUM BINDER CONTENT = 3.95 % ADDED 4.63 % TOTAL

DESIGN DATA AT OPTIMUM BINDER CONTENT:

Nmax

MIX VOIDS (%) 4.0 VMA (%) 13.4 VFB (%) 70.1 TH. MAX. SP. GR. (G_{mm}) 2.617 BULK SP. GR. (Gmb) 2.512 UNIT WEIGHT (lbs/ft3) 156.4 %G_{mm} @ N_{initial} 89.9 %G_{mm} @ N_{max} 96.7 TSR N= 23 81 RECOMMENDED MIXING TEMP. = 275-300 DUST TO Pbe RATIO 1.2

75

COMMENTS:

P-0.075 INCREASED 0.5% TO SIMULATE AGGREGATE BREAKDOWN DURING MIX PRODUCTION.

OMNNI ASSOCIATES

PAUL R. EGGEN

WISCONSIN CERTIFIED ASPHALTIC TECHNICIAN III

THE TEST DATA SHOWN ON THIS REPORT PERTAIN ONLY TO THE MATERIAL SUBMITTED FOR DESIGN PAGE 2



REPORT OFSUPERPAVE ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

E1-19.0 mm

1.0E+06

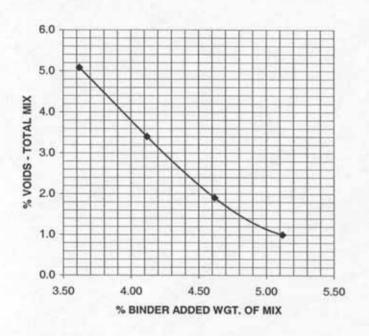
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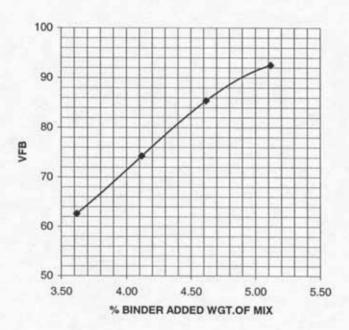
T0868A03-003

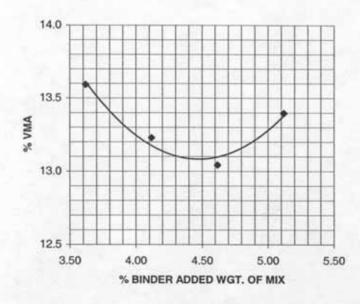
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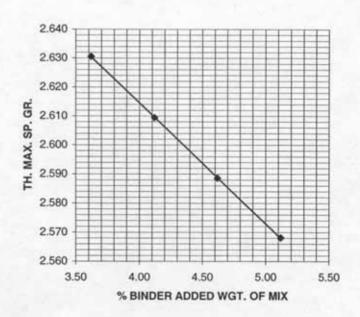
902903

DATE: 12/211/03











REPORT OFSUPERPAVE ASPHALTIC MIX DESIGN (AASHTO PP-28, ASTM D4867)

E1-19.0 mm

DESIGN NUMBER:

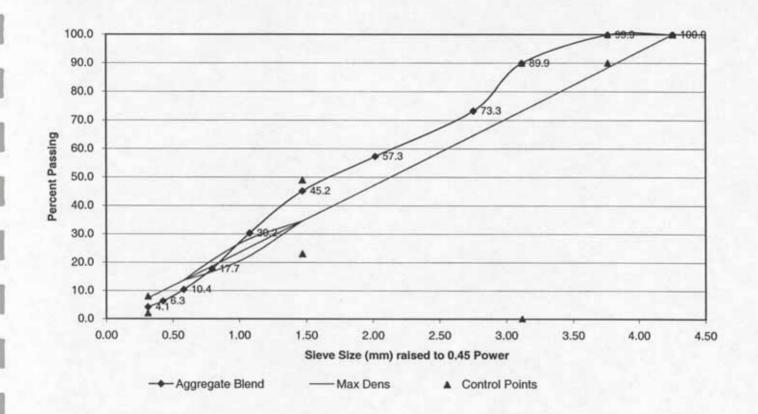
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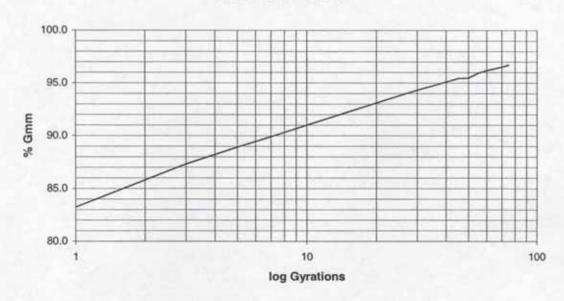
1.0E+06

JOB NUMBER T0868A03-003

DATE: 12/211/03



COMPACTION CURVE



Appendix D

Glass Aggregate Test Data



GLASS AGGREGATE PROPERTIES SUMMARY

Minergy, Prj. T0868A03-003 December 22, 2003

	Fox River Sediment Uncrushed Glass Aggregate	Fox River Sediment Crushed Glass Aggregate	Neenah Glass Aggregate
Gradation			
3/4" (19.0 mm)	100.0	100.0	100.0
1/2" (12.5 mm)	100.0	100.0	99.5
3/8" (9.5 mm)	100.0	100.0	93.3
#4 (4.75 mm)	99.9	100.0	52.9
#8 (2.36 mm)	79.6	96.6	19.3
#16 (1.18mm)	26.3	64.3	6.9
#30 (0.60mm)	7.9	29.2	2.6
#50 (0.30mm)	2.5	11.0	1.0
#100 (0.15mm)	0.7	3.1	0.4
#200 (0.075mm)	0.1	1.0	0.3
Fine Aggregate Angularity (%)	50.5	48.0	51.7
Bulk Specific Gravity (GSB)	2.878	2.886	2.787
Apparent Specific Gravity (GSA)	2.903	2.891	2.804
Absorption	0.30	0.06	0.60



OMNNI ASSOCIATES, INC. ONE SYSTEMS DRIVE APPLETON, WI 54914-1654 1-800-571-6677 920-735-6900 FAX 920-830-6100

REPORT OF: LABORATORY TESTS OF AGGREGATE

PROJECT: Fox River Glass Aggregate Mix Design and

OMNNI PROJECT NO. T0868A03

Testing

CLIENT:

Minergy

DATE:

12/11/03

Sample Number:

25390

Date of Sample:

8/03

Sample Source:

Minergy

Sample Location:

Pilot Plant - Winneconne

Tests Performed:

Grain Size Analysis, Specific Gravity, Fine Aggregate Angularity

TEST RESULTS;

Aggregate Description:

Crushed Fox River Sediment Glass Aggregate

Grain Size Analysis (ASTM:C136)

SIEVE SIZE	% PASSING	ASTM C33 SPECIFICATION
3/8"	100.0	100
#4	100.0	95-100
#8	96.6	80-100
#16	64.3	50-85
#30	29.2	25-60
#50	11.0	5-30
#100	3.1	0-10
#200	1.0	0-3

Specific Gravity and Absorption (ASTM: C128)

Bulk Specific Gravity	2.886
Apparent Specific Gravity	2.891
Absorption (%)	0.06

Fine Aggregate Angularity (ASTM: C1252, Method A)

Fine Aggregate Angularity (%)

48.0

REMARKS: The above sample was delivered to our laboratory by Minergy personnel.

Reviewed By:

Paul R Egger



OMNNI ASSOCIATES, INC. ONE SYSTEMS DRIVE APPLETON, WI 54914-1654 1-800-571-6677 920-735-6900 FAX 920-830-6100

REPORT OF: LABORATORY TESTS OF AGGREGATE

Fox River Glass Aggregate Mix Design and PROJECT:

OMNNI PROJECT NO. T0868A03

Testing

CLIENT:

Minergy

DATE: 12/11/03

Sample Number:

25490

Date of Sample:

8/03

Sample Source:

Minergy

Sample Location:

Pilot Plant - Winneconne

Tests Performed:

Grain Size Analysis, Specific Gravity, Fine Aggregate Angularity

TEST RESULTS;

Aggregate Description:

Fox River Sediment Glass Aggregate

Grain Size Analysis (ASTM:C136)

SIEVE SIZE	% PASSING
3/8"	100.0
#4	99.9
#8	79.6
#16	26.3
#30	7.9
#50	2.5
#100	0.7
#200	0.1

Specific Gravity and Absorption (ASTM: C128)

Bulk Specific Gravity	2.878
Apparent Specific Gravity	2.903
Absorption (%)	0.30

Fine Aggregate Angularity (ASTM: C1252, Method A)

50.5 Fine Aggregate Angularity (%)

REMARKS: The above sample was delivered to our laboratory by Minergy personnel.

Reviewed By:

ENGINEERING • ARCHITECTURE • ENVIRONMENTAL



One Systems Drive Appleton, WI 54914 1-800-571-6677 www.omnni.com